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October 12, 2017
Mr. Ralph Garcia
Senior Transportation Engineer
City of Sunnyvale
456 W. Olive Avenue
P.O Box 3707

Sunnyvale, CA 94008-3707
RE: LOS Results and Intersection Impacts for the Civic Center in Sunnyvale, CA

Dear Mr. Garcia:
The City of Sunnyvale is seeking to reconstruct and expand the existing Civic Center which includes the City Hall buildings, Public Safety Headquarters, and the Library located along Olive Avenue in Sunnyvale, CA.

After discussions with the City of Sunnyvale (City), this memorandum evaluated the intersection level of service (LOS) results and impacts. The following discusses the methodology, analysis, and results of the comparison.

## BACKGROUND

The proposed Civic Center is to be located on the northwest corner of the Mathilda Avenue/El Camino Real intersection, as shown in Figure 1. The existing City Hall buildings are 96,200 square feet and include the City Hall, Sunnyvale Office Center, South Annex, and City Hall Annex. The proposed new City Hall would be a maximum of 109,000 square feet (increase of 12,800 square feet). The existing Public Safety Headquarters is 41,000 square feet and the proposed building would be 65,000 square feet (increase of 24,000 square feet). The existing Library is 60,900 square feet and the proposed library would be 120,000 square feet (increase of 59,100 square feet). The City is considering two project options with different layouts for the buildings and parking structures, as shown in Figures 2 and 3. The second project option would close Olive Street in the middle of the project site, between Charles Street and Pastoria Avenue, which would affect the circulation of the proposed project traffic and the background traffic.

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Figure 1 - Project Location


Source: City of Sunnyvale

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Figure 2 - Project Option \#1


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Figure 3 - Project Option \#2


Source: City of Sunnyvale

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## Analysis Scenarios

To determine the potential project impacts, multiple scenarios were analyzed in the AM and PM peak hours:

- Existing Conditions - Based on the peak hour traffic counts provided by the City from November 2015, December 2016, and May 2017 and existing roadway geometry and traffic control.
- Existing Plus Project Option \#1 Traffic Conditions - Based on current traffic counts, existing roadway geometry and traffic control, plus traffic generated by the full buildout of Option \#1.
- Existing Plus Project Option \#2 Traffic Conditions - Based on current traffic counts, existing roadway geometry and traffic control, plus traffic generated by the full buildout of Option \#2.
- Existing + Background Traffic Conditions - Based on existing traffic volumes and traffic added by approved projects that the City has good reason to believe will be contributing traffic to the study area. The roadway network will include existing conditions plus programmed (i.e. funded) roadway projects to be in place by this analysis year.
- Existing + Background + Project Option \#1 Traffic Conditions - Based on existing traffic volumes, traffic added by approved projects as referenced above, and traffic generated by the full buildout of Option \#1. The scenario includes roadway projects programmed to be in place by this analysis year.
- Existing + Background + Project Option \#2 Traffic Conditions - Based on existing traffic volumes, traffic added by approved projects as referenced above, and traffic generated by the full buildout of Option \#2. The scenario includes roadway projects programmed to be in place by this analysis year.
- Cumulative (2025) Conditions without the Project - Based on existing traffic volumes and traffic added by approved and pending projects, as well as a 1.5 percent growth rate to all roadways. The roadway network will include existing conditions plus programmed (i.e. funded) roadway projects to be in place by this analysis year.
- Cumulative (2025) Conditions Plus Project Option \#1 - Based on Cumulative (2025) volumes and traffic generated by the full buildout of Option \#1.
- Cumulative (2025) Conditions Plus Project Option \#2 - Based on Cumulative (2025) volumes and traffic generated by the full buildout of Option \#2.


## Level of Service Standards

Analysis of significant environmental impacts at intersections and freeway segments is based on the concept of level of service (LOS). The LOS of an intersection is a qualitative measure used to describe operational conditions. LOS ranges from A (best), which represents minimal delay, to F (worst), which represents heavy delay and a facility that is operating at or near its functional capacity.

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Levels of service for this study were determined using methods defined in the Highway Capacity Manual, 2000 (HCM) and appropriate traffic analysis software.

The HCM includes procedures for analyzing side-street stop-controlled (SSSC), all-way stopcontrolled (AWSC), and signalized intersections. The SSSC procedure defines LOS as a function of average control delay for each minor street approach movement and major street left-turns. Conversely, the AWSC and signalized intersection procedures define LOS as a function of average control delay for the intersection as a whole. VTA has specific delay threshold values for each LOS that is more detailed than that of the HCM. Pluses and minuses are added to the HCM ranges to further designate the LOS for signalized intersections. Table 1 relates the operational characteristics associated with each LOS category for signalized intersections ${ }^{1}$

Table 2 relates the operational characteristics associated with each LOS category for unsignalized intersections ${ }^{2}$.

Table 1 - Signalized Intersection Level of Service Definitions

| Level of <br> Service | Description | Signalized <br> (Avg. control delay per <br> vehicle sec/veh.) |
| :--- | :--- | :--- |
| A | Free flow with no delays. Users are virtually unaffected <br> by others in the traffic stream | delay $\leq 10.0$ |
| B+ | Stable traffic. Traffic flows smoothly with few delays. | $10.0<$ delay $\leq 12.0$ |
| B | $12.0<$ delay $\leq 18.0$ |  |
| B- |  | $18.0<$ delay $\leq 20.0$ |
| C+ | Stable flow but the operation of individual users becomes | $20.0<$ delay $\leq 23.0$ |
| C | affected by other vehicles. Modest delays. | $32.0<$ delay $\leq 32.0$ |
| C- | Approaching unstable flow. Operation of individual users | $35.0<$ delay $\leq 35.0$ |
| D+ | becomes significantly affected by other vehicles. Delays | $39.0<$ delay $\leq 51.0$ |
| D | may be more than one cycle during peak hours. | $51.0<$ delay $\leq 55.0$ |
| D- | Unstable flow with operating conditions at or near the | $55.0<$ delay $\leq 60.0$ |
| E+ | $60.0<$ delay $\leq 75.0$ |  |
| E | capacity level. Long delays and vehicle queuing. | $75.0<$ delay $\leq 80.0$ |
| E- | Forced or breakdown flow that causes reduced capacity. | delay $>80$ |
| F | Stop and go traffic conditions. Excessive long delays |  |

${ }^{1}$ VTA Congestion M anagement Program, Traffic Level of Service Analysis Guidelines, June 2003.
${ }^{2}$ Transportation Research Board, Highway Capacity Manual 2000, National Research Council, 2000

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Table 2 - Unsignalized Intersection Level of Service Definitions

| Level of <br> Service | Description | Unsignalized <br> (Avg. control <br> delay per vehicle <br> sec/veh.) |
| :---: | :--- | :---: |
| A | Free flow with no delays. Users are virtually unaffected <br> by others in the traffic stream | $\leq 10$ |
| B | Stable traffic. Traffic flows smoothly with few delays. | $>10-15$ |
| C | Stable flow but the operation of individual users <br> becomes affected by other vehicles. Modest delays. | $>15-25$ |
| D | Approaching unstable flow. Operation of individual <br> users becomes significantly affected by other vehicles. <br> Delays may be more than one cycle during peak hours. | $>25-35$ |
| E | Unstable flow with operating conditions at or near the <br> capacity level. Long delays and vehicle queuing. | $>35-50$ |
| F | Forced or breakdown flow that causes reduced <br> capacity. Stop and go traffic conditions. Excessive <br> long delays and vehicle queuing. | $>50$ |

Project impacts were determined by comparing conditions with the proposed project to those without the proposed project. Significant impacts for signalized and unsignalized intersections are created when traffic from the proposed project causes the LOS to fall below a specific threshold. For unsignalized intersections, a deficient LOS suggests recommendations for improvements to the type of traffic control, such as signalization. A peak hour signal warrant was evaluated to determine if the intersection met the volume requirements for a traffic signal.

Consistent with the significance impact criteria documented in the Transportation Impact Analysis Guidelines ${ }^{3}$, VTA accepts a minimum level of service of LOS E for a County intersection or Congestion Management Program (CMP) intersection. The City utilizes the same VTA LOS standards for all intersections on regionally significant roadways such as El Camino Real. Therefore, the following conditions would result in a significant impact at a CMP intersection or regionally significant roadway intersection:

1. If the intersection operates at an acceptable LOS (i.e. LOS A, B, C, D, or E) without the project and degrades to an unacceptable LOS (i.e. LOS F) with the project, then it is a significant impact.
${ }^{3}$ Transportation Impact Analysis Guidelines, Santa Clara Valley Transportation Authority Guidelines, October 2014.

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2. If the intersection operates at an unacceptable LOS (i.e. LOS F) without the project and the project increases the average control delay for the critical movements by four (4) or more seconds and increases the critical volume to capacity ( $\mathrm{v} / \mathrm{c}$ ) by 0.01 or more, then it is a significant impact.
a. If the addition of project traffic reduces the amount of average control delay for a critical movement (i.e. negative change in delay) and the project increases the v/c by 0.01 or more, then it is a significant impact.

Mitigation for intersections with a significant impact must improve the LOS back to without Project conditions or better.

The LOS standard for City of Sunnyvale intersections is LOS D except for City of Sunnyvale intersections that are designated as regionally significant, which allows for a minimum level of service of LOS E. Therefore, the following conditions would result in a significant impact at a City intersection:

1. If the intersection operates at an acceptable LOS (i.e. LOS A, B, C, or D) without the project and degrades to an unacceptable LOS (i.e. LOS E, or F) with the project, then it is a significant impact.
2. If the intersection operates at an unacceptable LOS (i.e. LOS E, or F) without the project and the project increases the critical-movement delay of four (4) or more seconds and increased the critical volume to capacity $(\mathrm{v} / \mathrm{c})$ by 0.01 or more, then it is a significant impact.

The City of Sunnyvale does not have an officially adopted significance criterion for unsignalized intersections. Based on previously approved traffic studies, significant impacts are defined to occur when:

1. The addition of project traffic causes the average intersection delay for all-way stop controlled intersections or the worst movement/approach for side-street stop-controlled intersections to degrade to LOS E or LOS F for regionally significant roadways, respectively.
2. The intersection satisfies any traffic signal warrant from the MUTCD.

Mitigation for intersections with a significant impact must improve the LOS back to without Project Conditions or better.

## Analysis Methodology

The study intersections were modeled in Traffix software using Highway Capacity Manual (HCM) 2000 methodology for the AM and PM peak periods, consistent with VTA methodology. The LOS were reported for each study intersection.

## EXISTING CONDITIONS

The existing conditions for the study intersections were evaluated in Traffix using the existing volumes and lane geometry. Table 3 shows the existing LOS and delay for each of the study intersections. The LOS calculations are attached.

Table 3 - Existing LOS Summary

| \# | Intersection | LOS <br> Criteria | Jurisdiction | Control | Existing |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AM Peak |  |  |  | PM Peak |  |  |  |
|  |  |  |  |  | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Crit. <br> Delay | LOS | Delay $(\sec )^{1}$ | v/c ratio | Crit. <br> Delay |
| 1 | Mathilda Avenue/WB SR-237 Ramps | E | City | Signal | C | 23.5 | 0.500 | 19.7 | C+ | 23.0 | 0.565 | 26.0 |
| 2 | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | C | 25.9 | 0.393 | 27.7 | B- | 19.0 | 0.421 | 9.5 |
| 3 | Mathilda Avenue/Ross Drive | E | City | Signal | C+ | 20.4 | 0.383 | 16.1 | C+ | 20.2 | 0.446 | 13.8 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | C | 24.2 | 0.506 | 24.9 | C | 30.1 | 0.455 | 24.4 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | A | 8.5 | 0.534 | 9.1 | B | 13.8 | 0.384 | 9.5 |
| 6 | Mathilda Avenue/Maude Avenue | E | City | Signal | D | 41.4 | 0.657 | 39.0 | D | 48.5 | 0.660 | 47.6 |
| 7 | Mathilda Avenue/Indio Way | E | City | Signal | C | 30.2 | 0.636 | 28.8 | C | 26.3 | 0.724 | 27.4 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | C | 23.8 | 0.482 | 12.4 | C | 31.1 | 0.747 | 30.2 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | C | 31.9 | 0.690 | 32.8 | C- | 32.8 | 0.709 | 27.8 |
| 10 | Mathilda Avenue/McKinley Avenue | E | City | Signal | B | 13.1 | 0.465 | 12.7 | B- | 18.6 | 0.568 | 18.4 |
| 11 | Mathilda Avenue/lowa Avenue | E | City | Signal | B | 15.7 | 0.438 | 11.5 | B- | 19.4 | 0.492 | 15.8 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B- | 18.9 | 0.624 | 15.3 | B- | 18.1 | 0.530 | 14.2 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Caltrans | Signal | D- | 53.3 | 0.780 | 55.9 | D | 49.1 | 0.743 | 50.4 |
| 14 | Sunnyvale Avenue/EI Camino Real | E | City / Caltrans | Signal | D+ | 37.7 | 0.393 | 34.8 | D | 44.2 | 0.595 | 44.4 |
| 15 | Mathilda Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | C | 23.8 | 0.528 | 22.3 | C | 31.6 | 0.525 | 30.5 |
| 16 | Sunnyvale-Saratoga Road/Remington Drive | E | City | Signal | D | 41.3 | 0.784 | 35.7 | D | 45.7 | 0.751 | 48.0 |
| 17 | Sunnyvale-Saratoga Road/Fremont Avenue | E | City | Signal | D | 48.3 | 0.789 | 46.6 | D | 49.1 | 0.756 | 47.6 |
| 18 | S Pastoria Avenue/W Washington Avenue | D | City | Signal | B | 13.1 | 0.282 | 13.5 | B | 13.4 | 0.326 | 13.8 |
| 19 | S Pastoria Avenue/W lowa Avenue | D | City | Signal | A | 9.5 | 0.234 | 9.0 | B+ | 10.4 | 0.288 | 9.9 |
| 20 | S Pastoria Avenue/W Olive Avenue | D | City | ASWC | A | 9.7 | 0.314 | 9.7 | B | 10.2 | 0.323 | 10.2 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | D+ | 38.3 | 0.526 | 35.7 | D+ | 38.0 | 0.605 | 38.0 |
| 22 | Charles Street/W lowa Avenue | D | City | SSSC | A | 10.0 | 0.022 | 2.7 | B | 10.9 | 0.041 | 2.7 |
| 23 | Mary Avenue/W Olive Avenue | D | City | SSSC | D | 33.8 | 0.175 | 2.5 | E | 41.5 | 0.260 | 2.6 |
| 24 | Mary Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.8 | 0.652 | 41.3 | D | 44.6 | 0.736 | 44.3 |
| 25 | Sunnyvale-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | A | 9.4 | 0.540 | 6.9 | A | 8.4 | 0.473 | 7.2 |
| 26 | Sunnyvale-Saratoga Road/Alberta Avenue-Harwick Way | E | City | Signal | C+ | 22.8 | 0.615 | 18.0 | C | 23.7 | 0.569 | 22.7 |
| 27 | Sunnyvale-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | D+ | 39.0 | 0.796 | 39.2 | D | 40.8 | 0.811 | 46.1 |

Note: Locations operating unacceptably are in bold.
All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (PM peak hour only)


## TRIP GENERATION AND DISTRIBUTION

The trip generation, as shown in the Initial Coordination and Trip Assumptions for the Civic Center in Sunnyvale, CA memorandum dated October 3, 2017, showed two separate methodologies for determining the estimated number of project trips. One method used the Institute of Transpiration Engineers (ITE) Trip Generation Manual to estimate the project trips. The second method used traffic counts of existing trips to the Sunnyvale Civic Center. AM peak hour and PM peak hour counts were collected on Tuesday, 5/9/2017, Wednesday, 5/10/2017, and Tuesday, 5/23/2017, and an average of the three days was used. Upon further review of the trip generation rates in the ITE Trip Generation Manual, the assumed land use of a government building has much different in and out splits in the AM peak hour and in the PM peak hour. Therefore, it was determined that the existing counts were a better representation of future trips generated. Table 4 shows the trip generation for the proposed land use expansions using site counts. The proposed project will generate an estimated 120 AM peak hour trips and 255 PM peak hour trips.

The project trip distribution for the proposed project is shown in Figure 4. The trip distribution for the government land uses was determined based on discussions with City staff. The trip distribution for the library is similar but assumes more trips coming from within the City of Sunnyvale.

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Table 4 - Trip Generation Summary (Site Counts)

| TIME PERIOD |  | LAND USE | Trips |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | In | Out | Total |
| AM Peak | Existing | City Hall (96.2 KSF) | 194 | 44 | 238 |
|  |  | Public Safety Headquarters (41 KSF) | 31 | 23 | 54 |
|  |  | Library ( 60.9 KSF ) | 39 | 20 | 59 |
|  |  | Total Existing Trips | 264 | 87 | 351 |
|  | Project | City Hall (109 KSF) | 220 | 50 | 270 |
|  |  | Public Safety Headquarters ( 65 KSF) | 49 | 36 | 85 |
|  |  | Library (120 KSF) | 77 | 39 | 116 |
|  |  | Total Proposed Trips | 346 | 126 | 471 |
|  |  | Net New Trips | 82 | 39 | 120 |
| PM Peak | Existing | City Hall (96.2 KSF) | 53 | 163 | 216 |
|  |  | Public Safety Headquarters (41 KSF) | 21 | 50 | 71 |
|  |  | Library ( 60.9 KSF ) | 94 | 97 | 191 |
|  |  | Total Existing Trips | 168 | 310 | 478 |
|  | Project | City Hall (109 KSF) | 60 | 185 | 245 |
|  |  | Public Safety Headquarters ( 65 KSF ) | 33 | 79 | 112 |
|  |  | Library (120 KSF) | 185 | 191 | 376 |
|  |  | Total Proposed Trips | 278 | 455 | 733 |
|  |  | Net New Trips | 110 | 145 | 255 |

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Figure 4 - Project Trip Distribution

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It should be noted that a significant proportion of the trips go to and come from Mathilda Avenue to the north. These trips are going to or coming from Central Expressway, US-101, SR-237, and Moffett Park. There are approximately 223 vehicles in the AM peak hour and 303 vehicles in the PM peak hour to/from the north.

## EXISTING PLUS PROJECT CONDITIONS

## Existing Plus Project - Option 1

Existing Plus Project (Option 1) volumes were generated by adding the project trips based on the trip generation and trip distribution assumptions at the study intersections with West Olive Avenue as an access way to the proposed project site. Table 5 shows the Existing Plus Project (Option 1) LOS and delay for each of the study intersections. The LOS calculations are attached.

All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (AM and PM peak hours)

This is a significant impact in the AM peak hour because the project increases the LOS from an acceptable LOS D to an unacceptable LOS E.

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Table 5 - Existing Plus Project (Option 1) LOS Summary

| \# | Intersection | $\begin{aligned} & \text { LOS } \\ & \text { Criteria } \end{aligned}$ | Jurisdiction | Control | Existing |  |  |  |  |  |  |  | Existing + Project (Option 1) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AMPeak |  |  |  | PMPeak |  |  |  | AM Peak |  |  |  |  |  | PM Peak |  |  |  |  |  |
|  |  |  |  |  | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | vc ratio | Critica Delay <br> (sec) | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | $\begin{array}{\|l\|l\|} \hline \text { Cricital } \\ \text { Celay } \end{array}$ | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Var | Critical Delay <br> (sec) | Var | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | V/c ratio | Var | Critical Delay (sec) | Var |
| 1 | Mathilda Avenue/WB SR-237 Ramps | E | City | Signal | C | 23.5 | 0.500 | 19.7 | C+ | 23.0 | 0.565 | 26.0 | c | 23.6 | 0.500 | 0.000 | 19.6 | -0.1 | C+ | 22.9 | 0.565 | 0.000 | 25.9 | -0.1 |
| 2 | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | c | 25.9 | 0.393 | 27.7 | B- | 19.0 | 0.421 | 9.5 | c | 25.9 | 0.394 | 0.001 | 27.7 | 0.0 | B- | 19.0 | 0.424 | 0.003 | 9.4 | -0.1 |
| 3 | Mathilda Avenue/Ross Drive | E | City | Signal | ${ }^{\text {C+ }}$ | 20.4 | 0.383 | 16.1 | ${ }^{+}+$ | 20.2 | 0.446 | 13.8 | ${ }^{\text {C+ }}$ | 20.4 | 0.384 | 0.001 | 16.1 | 0.0 | ${ }_{\text {C+ }}$ | 20.1 | 0.449 | 0.003 | 13.8 | 0.0 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | c | 24.2 | 0.506 | 24.9 | c | 30.1 | 0.455 | 24.4 | c | 24.1 | 0.508 | 0.002 | 24.8 | -0.1 | C | 29.9 | 0.459 | 0.004 | 24.3 | -0.1 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | A | 8.5 | 0.534 | 9.1 | B | 13.8 | 0.384 | 9.5 | A | 8.5 | 0.536 | 0.002 | 9.1 | 0.0 | B | 14.4 | 0.396 | 0.012 | 20.6 | 11.1 |
| 6 | Mathida Avenue/Maude Avenue | E | City | Signal | D | 41.4 | 0.657 | 39.0 | D | 48.5 | 0.660 | 47.6 | D | 41.4 | 0.659 | 0.002 | 39.0 | 0.0 | D | 49.1 | 0.650 | -0.010 | 55.2 | 7.6 |
| 7 | Mathilda Avenue/Indio Way | E | City | Signal | C | 30.2 | 0.636 | 28.8 | C | 26.3 | 0.724 | 27.4 | c | 30.1 | 0.638 | 0.002 | 28.7 | -0.1 | C | 26.4 | 0.732 | 0.008 | 27.7 | 0.3 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | c | 23.8 | 0.482 | 12.4 | c | 31.1 | 0.747 | 30.2 | c | 23.7 | 0.485 | 0.003 | 12.3 | -0.1 | c | 31.3 | 0.756 | 0.009 | 30.5 | 0.3 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | c | 31.9 | 0.690 | 32.8 | C- | 32.8 | 0.709 | 27.8 | c | 31.7 | 0.693 | 0.003 | 32.8 | 0.0 | C- | 32.7 | 0.716 | 0.007 | 27.8 | 0.0 |
| 10 | Mathilda Avenue/McKinley Avenue | E | City | Signal | B | 13.1 | 0.465 | 12.7 | B- | 18.6 | 0.568 | 18.4 | B | 13.3 | 0.470 | 0.005 | 13.1 | 0.4 | B- | 18.6 | 0.579 | 0.011 | 18.7 | 0.3 |
| 11 | Mathilda Avenue/lowa Avenue | E | City | Signal | B | 15.7 | 0.438 | 11.5 | B- | 19.4 | 0.492 | 15.8 | B | 17.1 | 0.448 | 0.010 | 13.2 | 1.7 | C+ | 21.4 | 0.511 | 0.019 | 15.7 | -0.1 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B- | 18.9 | 0.624 | 15.3 | B- | 18.1 | 0.530 | 14.2 | B- | 19.6 | 0.629 | 0.005 | 15.6 | 0.3 | C+ | 20.4 | 0.568 | 0.038 | 18.8 | 4.6 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Caltrans | Signal | D- | 53.3 | 0.780 | 55.9 | D | 49.1 | 0.743 | 50.4 | D- | 53.9 | 0.790 | 0.010 | 56.6 | 0.7 | D | 48.9 | 0.741 | -0.002 | 49.9 | -0.5 |
| 14 | Sunnyvale Avenue/EI Camino Real | E | City / Caltrans | Signal | D+ | 37.7 | 0.393 | 34.8 | D | 44.2 | 0.595 | 44.4 | D+ | 37.6 | 0.394 | 0.001 | 34.7 | -0.1 | D | 44.0 | 0.597 | 0.002 | 44.4 | 0.0 |
| 15 | Mathida Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | c | 23.8 | 0.528 | 22.3 | c | 31.6 | 0.525 | 30.5 | c | 23.7 | 0.531 | 0.003 | 22.3 | 0.0 | c | 31.4 | 0.530 | 0.005 | 30.5 | 0.0 |
| 16 | Sunnyval-Saratoga Road/Remington Drive | E | City | Signal | D | 41.3 | 0.784 | 35.7 | D | 45.7 | 0.751 | 48.0 | D | 41.5 | 0.787 | 0.003 | 35.7 | 0.0 | D | 45.8 | 0.759 | 0.008 | 48.3 | 0.3 |
| 17 | Sunnyval-Saratoga Road/Fremont Avenue | E | City | Signal | D | 48.3 | 0.789 | 46.6 | D | 49.1 | 0.756 | 47.6 | D | 48.3 | 0.793 | 0.004 | 46.7 | 0.1 | D | 49.0 | 0.763 | 0.007 | 47.7 | 0.1 |
| 18 | S Pastoria Avenue/W Washington Avenue | D | City | Signal | B | 13.1 | 0.282 | 13.5 | B | 13.4 | 0.326 | 13.8 | B | 13.1 | 0.282 | 0.000 | 13.5 | 0.0 | B | 13.4 | 0.326 | 0.000 | 13.8 | 0.0 |
| 19 | S Pastoria Avenue/W lowa Avenue | D | City | Signal | A | 9.5 | 0.234 | 9.0 | B+ | 10.4 | 0.288 | 9.9 | A | 9.5 | 0.234 | 0.000 | 9.0 | 0.0 | B+ | 10.4 | 0.288 | 0.000 | 9.9 | 0.0 |
| 20 | S Pastoria Avenue/W Olive Avenue | D | City | ASWC | A | 9.7 | 0.314 | 9.7 | B | 10.2 | 0.323 | 10.2 | A | 9.8 | 0.319 | 0.005 | 9.8 | 0.1 | B | 10.3 | 0.334 | 0.011 | 10.3 | 0.1 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | ${ }_{\text {D }}+$ | 38.3 | 0.526 | 35.7 | D+ | 38.0 | 0.605 | 38.0 | $\mathrm{D}_{+}$ | 38.3 | 0.528 | 0.002 | 35.7 | 0.0 | D+ | 38.1 | 0.609 | 0.004 | 38.3 | 0.3 |
| 22 | Charles StreetW lowa Avenue | D | City | Sssc | A | 10.0 | 0.022 | 2.7 | B | 10.9 | 0.041 | 2.7 | B | 10.8 | 0.040 | 0.018 | 4.0 | 1.3 | B | 13.0 | 0.090 | 0.049 | 4.8 | 2.1 |
| 23 | Mary Avenue/W Olive Avenue | D | City | SSSC | D | 33.8 | 0.175 | 2.5 | E | 41.5 | 0.260 | 2.6 | E | 35.3 | 0.195 | 0.020 | 2.6 | 0.1 | E | 43.9 | 0.263 | 0.003 | 3.0 | 0.4 |
| 24 | Mary Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.8 | 0.652 | 41.3 | D | 44.6 | 0.736 | 44.3 | D | 43.0 | 0.656 | 0.004 | 41.5 | 0.2 | D | 45.2 | 0.747 | 0.011 | 45.0 | 0.7 |
| 25 | Sunnyval-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | A | 9.4 | 0.540 | 6.9 | A | 8.4 | 0.473 | 7.2 | A | 9.4 | 0.544 | 0.004 | 6.9 | 0.0 | A | 8.3 | 0.480 | 0.007 | 7.1 | -0.1 |
| 26 | Sunnyval-Saratoga Road/Alberta Avenue-Harwick Way | E | City | Signal | C+ | 22.8 | 0.615 | 18.0 | c | 23.7 | 0.569 | 22.7 | C+ | 22.7 | 0.619 | 0.004 | 18.0 | 0.0 | c | 23.5 | 0.576 | 0.007 | 22.6 | -0.1 |
| 27 | Sunnyval-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | D+ | 39.0 | 0.796 | 39.2 | D | 40.8 | 0.811 | 46.1 | D | 39.2 | 0.801 | 0.005 | 39.3 | 0.1 | D | 41.0 | 0.820 | 0.009 | 46.5 | 0.4 |

## Kimley»"Horn

## Existing Plus Project - Option 2

Existing Plus Project (Option 2) volumes were generated by adding the project trips based on the trip generation and trip distribution assumptions at the study intersections with no access to the proposed project site through West Olive Avenue. Table 6 shows the Existing Plus Project (Option 2) LOS and delay for each of the study intersections. The LOS calculations are attached.

All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (AM and PM peak hours)

This is a significant impact in the AM peak hour because the project increases the LOS from an acceptable LOS D to an unacceptable LOS E.

Table 6 - Existing Plus Project (Option 2) LOS Summary

|  | Intersection | $\begin{aligned} & \text { LOS } \\ & \text { Criteria } \end{aligned}$ | Juriscicition | Control | Existing |  |  |  |  |  |  |  | Existing + Project (Option 2) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AMPeak |  |  |  | PM Peak |  |  |  | AM Peak |  |  |  |  |  | PM Peak |  |  |  |  |  |
| \# |  |  |  |  | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | V/c ratio | $\begin{aligned} & \text { Critical } \\ & \text { Deflay } \end{aligned}$ $(\mathrm{sec})$ | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | V/c ratio | $\begin{aligned} & \text { Cricital } \\ & \text { Celay } \end{aligned}$ | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Var | $\begin{aligned} & \text { Critical } \\ & \text { Deflay } \end{aligned}$ $(\mathrm{sec})$ | Var | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Var | Ciritical Delay (sec) (sec | Var |
| 1 | Mathilda Avenue/WB SR-237 Ramps | E | City | Signal | c | 23.5 | 0.500 | 19.7 | C+ | 23.0 | 0.565 | 26.0 | C | 23.6 | 0.500 | 0.000 | 19.6 | -0.1 | C+ | 22.9 | 0.565 | 0.000 | 25.9 | -0.1 |
|  | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | c | 25.9 | 0.393 | 27.7 | B- | 19.0 | 0.421 | 9.5 | c | 25.9 | 0.394 | 0.001 | 27.7 | 0.0 | B- | 19.0 | 0.424 | 0.003 | 9.4 | -0.1 |
| 3 | Mathilda Avenue/Ross Drive | E | City | Signal | ${ }_{\text {C+ }}$ | 20.4 | 0.383 | 16.1 | ${ }_{+}^{+}$ | 20.2 | 0.446 | 13.8 | ${ }_{\text {C+ }}$ | 20.4 | 0.384 | 0.001 | 16.1 | 0.0 | C+ | 20.1 | 0.449 | 0.003 | 13.8 | 0.0 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | c | 24.2 | 0.506 | 24.9 | C | 30.1 | 0.455 | 24.4 | c | 24.1 | 0.508 | 0.002 | 24.8 | -0.1 | C | 29.9 | 0.459 | 0.004 | 24.3 | -0.1 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | A | 8.5 | 0.534 | 9.1 | B | 13.8 | 0.384 | 9.5 | A | 8.5 | 0.536 | 0.002 | 9.1 | 0.0 | B | 14.4 | 0.396 | 0.012 | 20.6 | 11.1 |
| 6 | Mathilda Avenue/Maude Avenue | E | City | Signal | D | 41.4 | 0.657 | 39.0 | D | 48.5 | 0.660 | 47.6 | D | 41.4 | 0.659 | 0.002 | 39.0 | 0.0 | D | 49.1 | 0.650 | -0.010 | 55.2 | 7.6 |
| 7 | Mathilda Avenue/Indio Way | E | City | Signal | c | 30.2 | 0.636 | 28.8 | c | 26.3 | 0.724 | 27.4 | C | 30.1 | 0.638 | 0.002 | 28.7 | $-0.1$ | C | 26.4 | 0.732 | 0.008 | 27.7 | 0.3 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | c | 23.8 | 0.482 | 12.4 | c | 31.1 | 0.747 | 30.2 | c | 23.7 | 0.485 | 0.003 | 12.3 | -0.1 | c | 31.3 | 0.756 | 0.009 | 30.5 | 0.3 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | c | 31.9 | 0.690 | 32.8 | C- | 32.8 | 0.709 | 27.8 | c | 31.7 | 0.693 | 0.003 | 32.8 | 0.0 | C- | 32.7 | 0.716 | 0.007 | 27.8 | 0.0 |
| 10 | Mathilda Avenue/McKinley Avenue | E | City | Signal | B | 13.1 | 0.465 | 12.7 | B- | 18.6 | 0.568 | 18.4 | B | 13.4 | 0.470 | 0.005 | 13.2 | 0.5 | B- | 19.7 | 0.589 | 0.021 | 20.0 | 1.6 |
| 11 | Mathilda Avenue/lowa Avenue | E | City | Signal | B | 15.7 | 0.438 | 11.5 | B- | 19.4 | 0.492 | 15.8 | B | 16.8 | 0.446 | 0.008 | 12.9 | 1.4 | C+ | 21.8 | 0.514 | 0.022 | 16.2 | 0.4 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B- | 18.9 | 0.624 | 15.3 | B- | 18.1 | 0.530 | 14.2 | B- | 19.9 | 0.628 | 0.004 | 15.6 | 0.3 | B | 17.9 | 0.540 | 0.010 | 15.5 | 1.3 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Caltrans | Signal | D- | 53.3 | 0.780 | 55.9 | D | 49.1 | 0.743 | 50.4 | D- | 53.5 | 0.788 | 0.008 | 56.3 | 0.4 | D | 49.1 | 0.740 | -0.003 | 50.2 | -0.2 |
| 14 | Sunnyvale Avenue/EI Camino Real | E | City / Caltrans | Signal | D+ | 37.7 | 0.393 | 34.8 | D | 44.2 | 0.595 | 44.4 | D+ | 37.6 | 0.394 | 0.001 | 34.7 | -0.1 | D | 44.0 | 0.597 | 0.002 | 44.4 | 0.0 |
| 15 | Mathilda Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | c | 23.8 | 0.528 | 22.3 | c | 31.6 | 0.525 | 30.5 | c | 23.9 | 0.522 | -0.006 | 22.4 | 0.1 | c | 31.7 | 0.522 | ${ }^{-0.003}$ | 30.6 | 0.1 |
| 16 | Sunnyval-Saratoga Road/Remington Drive | E | City | Signal | D | 41.3 | 0.784 | 35.7 | D | 45.7 | 0.751 | 48.0 | D | 41.3 | 0.778 | -0.006 | 35.7 | 0.0 | D | 45.7 | 0.748 | ${ }^{-0.003}$ | 48.0 | 0.0 |
| 17 | Sunnyval-Saratoga Road/Fremont Avenue | E | City | Signal | D | 48.3 | 0.789 | 46.6 | D | 49.1 | 0.756 | 47.6 | D | 48.3 | 0.783 | -0.006 | 46.6 | 0.0 | D | 49.1 | 0.753 | -0.003 | 47.6 | 0.0 |
| 18 | S Pastoria Avenue $W$ Washington Avenue | D | City | Signal | B | 13.1 | 0.282 | 13.5 | B | 13.4 | 0.326 | 13.8 | B | 13.1 | 0.282 | 0.000 | 13.5 | 0.0 | B | 13.4 | 0.326 | 0.000 | 13.8 | 0.0 |
| 19 | S Pastoria AvenueN lowa Avenue | D | City | Signal | A | 9.5 | 0.234 | 9.0 | B+ | 10.4 | 0.288 | 9.9 | B+ | 10.0 | 0.248 | 0.014 | 10.1 | 1.1 | B+ | 11.4 | 0.321 | 0.033 | 12.0 | 2.1 |
| 20 | S Pastoria Avenue/ O Olive Avenue | D | City | ASWC | A | 9.7 | 0.314 | 9.7 | B | 10.2 | 0.323 | 10.2 | B | 10.1 | 0.349 | 0.035 | 10.1 | 0.4 | B | 10.6 | 0.395 | 0.072 | 10.6 | 0.4 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | ${ }_{\text {D }}+$ | 38.3 | 0.526 | 35.7 | D+ | 38.0 | 0.605 | 38.0 | D | 39.2 | 0.554 | 0.028 | 37.1 | 1.4 | D | 39.2 | 0.621 | 0.016 | 39.1 | 1.1 |
| 22 | Charles StreetW Iowa Avenue | D | City | sssc | A | 10.0 | 0.022 | 2.7 | B | 10.9 | 0.041 | 2.7 | B | 10.2 | 0.032 | 0.010 | 2.7 | 0.0 | B | 12.1 | 0.072 | 0.031 | 2.9 | 0.2 |
| 23 | Mary Avenue/W Olive Avenue | D | City | sssc | D | 33.8 | 0.175 | 2.5 | E | 41.5 | 0.260 | 2.6 | E | 35.3 | 0.195 | 0.020 | 2.6 | 0.1 | E | 43.9 | 0.263 | 0.003 | 3.0 | 0.4 |
| 24 | Mary Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.8 | 0.652 | 41.3 | D | 44.6 | 0.736 | 44.3 | D | 43.0 | 0.656 | 0.004 | 41.5 | 0.2 | D | 45.2 | 0.747 | 0.011 | 45.0 | 0.7 |
| 25 | Sunnyval-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | A | 9.4 | 0.540 | 6.9 | A | 8.4 | 0.473 | 7.2 | A | 9.4 | 0.534 | -0.006 | 7.0 | 0.1 | A | 8.4 | 0.471 | -0.002 | 7.2 | 0.0 |
| 26 | Sunnyval-Saratoga RoadAliberta Avenue-Harwick Way | E | City | Signal | ${ }_{\text {C+ }}$ | 22.8 | 0.615 | 18.0 | c | 23.7 | 0.569 | 22.7 | ${ }_{\text {C+ }}$ | 22.9 | 0.609 | -0.006 | 18.0 | 0.0 | C | 23.7 | 0.566 | ${ }^{-0.003}$ | 22.8 | 0.1 |
| 27 | Sunnyval-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | D+ | 39.0 | 0.796 | 39.2 | D | 40.8 | 0.811 | 46.1 | D+ | 38.9 | 0.790 | $-0.006$ | 39.1 | -0.1 | D | 40.8 | 0.808 | $-0.003$ | 46.1 | 0.0 |
| Note: | The average control delay is reported for signalized and AWSC int Locations operating unacceptably are in bold and impacts are high | $\overline{\text { ns. Th }}$ | ay for the wor | ovemen | repor | for SSS | intersec | tions. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## EXISTING PLUS BACKGROUND CONDITIONS

To achieve Existing Plus Background traffic conditions, traffic volumes from approved but not yet constructed projects were incorporated according to the information provided by the City (dated September 2017).

Table 7 shows the Existing Plus Approved LOS and delay for each of the study intersections. The LOS calculations are attached.

Table 7 - Existing Plus Background LOS Summary

| \# | Intersection | LOS <br> Criteria | Jurisdiction | Control | Background |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AMPeak |  |  |  | PMPeak |  |  |  |
|  |  |  |  |  | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Crit. <br> Delay | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \\ & \hline \end{aligned}$ | v/c ratio | Crit. <br> Delay |
| 1 | Mathilda Avenue/WB SR-237 Ramps | E | City | Signal | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.0 |
| 2 | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | D+ | 37.0 | 0.515 | 38.8 | C | 23.5 | 0.484 | 12.3 |
| 3 | Mathilda Avenue/Ross Drive | E | City | Signal | C | 25.7 | 0.428 | 20.8 | C | 25.0 | 0.500 | 16.2 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | C | 26.2 | 0.574 | 24.9 | C- | 32.2 | 0.555 | 25.4 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | A | 8.3 | 0.618 | 9.5 | B | 13.3 | 0.464 | 19.2 |
| 6 | Mathilda Avenue/Maude Avenue | E | City | Signal | D | 49.1 | 0.828 | 67.7 | D- | 55.0 | 0.800 | 57.1 |
| 7 | Mathilda Avenue/Indio Way | E | City | Signal | D+ | 38.7 | 0.771 | 39.2 | C- | 34.1 | 0.856 | 37.8 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | C | 27.4 | 0.647 | 20.3 | C- | 34.7 | 0.840 | 34.8 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | D+ | 38.3 | 0.827 | 40.2 | D+ | 38.4 | 0.810 | 34.2 |
| 10 | Mathilda Avenue/McKinley Avenue | E | City | Signal | B | 15.0 | 0.668 | 14.9 | B- | 18.2 | 0.619 | 17.0 |
| 11 | Mathilda Avenue/lowa Avenue | E | City | Signal | B | 17.9 | 0.622 | 13.6 | B- | 18.3 | 0.533 | 11.8 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B | 16.7 | 0.624 | 11.2 | C+ | 20.4 | 0.587 | 15.3 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Caltrans | Signal | E+ | 57.1 | 0.796 | 59.4 | E+ | 57.3 | 0.836 | 61.4 |
| 14 | Sunnyvale Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.1 | 0.427 | 36.6 | D | 48.9 | 0.712 | 52.5 |
| 15 | Mathilda Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | C | 26.8 | 0.583 | 26.1 | C- | 33.3 | 0.569 | 32.7 |
| 16 | Sunnyvale-Saratoga Road/Remington Drive | E | City | Signal | D | 46.3 | 0.848 | 42.6 | D | 49.3 | 0.790 | 51.1 |
| 17 | Sunnyvale-Saratoga Road/Fremont Avenue | E | City | Signal | D- | 52.9 | 0.838 | 52.2 | D- | 53.5 | 0.806 | 52.4 |
| 18 | S Pastoria Avenue/W Washington Avenue | D | City | Signal | B | 13.6 | 0.333 | 13.9 | B | 14.2 | 0.411 | 14.6 |
| 19 | S Pastoria Avenue/W lowa Avenue | D | City | Signal | A | 9.1 | 0.280 | 9.2 | B+ | 10.6 | 0.346 | 10.6 |
| 20 | S Pastoria Avenue/W Olive Avenue | D | City | ASWC | B | 10.9 | 0.372 | 10.9 | B | 12.5 | 0.434 | 12.5 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.2 | 0.524 | 38.7 | D | 42.3 | 0.628 | 43.0 |
| 22 | Charles Street/W lowa Avenue | D | City | SSSC | B | 10.1 | 0.023 | 2.5 | B | 11.2 | 0.043 | 2.5 |
| 23 | Mary Avenue/W Olive Avenue | D | City | SSSC | F | 77.3 | 0.307 | 4.4 | F | 89.3 | 0.394 | 4.6 |
| 24 | Mary Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 50.1 | 0.732 | 50.3 | D- | 52.2 | 0.784 | 51.6 |
| 25 | Sunnyvale-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | B | 12.7 | 0.600 | 10.8 | B+ | 10.4 | 0.521 | 9.6 |
| 26 | Sunnyvale-Saratoga Road/Alberta Avenue-Harwick Way | E | City | Signal | C | 26.4 | 0.671 | 23.0 | C | 25.7 | 0.612 | 25.4 |
| 27 | Sunnyvale-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | D | 47.8 | 0.930 | 53.0 | D | 50.4 | 0.924 | 57.3 |

Note: Locations operating unacceptably are in bold.
All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (AM and PM peak hours)


## EXISTING PLUS BACKGROUND PLUS PROJECT CONDITIONS

## Existing Plus Background Plus Project - Option 1

Existing Plus Background Plus Project (Option 1) volumes were generated by adding the project trips to the Existing Plus Background volumes. Table 8 shows the Existing Plus Background Plus Project LOS and delay for each of the study intersections. The LOS calculations are attached.

Table 8 - Existing Plus Background Plus Project (Option 1) LOS Summary

| \# | Intersection | $\begin{aligned} & \text { LOS } \\ & \text { Criteria } \end{aligned}$ | Juriscicition | Control | Background |  |  |  |  |  |  |  | Background + Project (Option 1) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AM Peak |  |  |  | PMPeak |  |  |  | AM Peak |  |  |  |  |  | PM Peak |  |  |  |  |  |
|  |  |  |  |  | LOS | $\begin{aligned} & \text { Deay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/cratio | $\begin{aligned} & \text { Crit. } \\ & \text { Delay } \end{aligned}$ | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | $\begin{aligned} & \text { Crit. } \\ & \text { Delay } \end{aligned}$ | LOS | $\begin{aligned} & \text { Deay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | vc ratio | Var | Critica Delay (sec) | Var | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | vch ratio | Var | Critical Delay (sec) | Var |
| 1 | Mathilda Avenue-WB SR-237 Ramps | E | City | Signal | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.000 | 0.0 | 0.0 | A | 0.00 | 0.000 | 0.000 | 0.0 | 0.0 |
| 2 | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | D+ | 37.0 | 0.515 | 38.8 | c | 23.5 | 0.484 | 12.3 | D+ | 37.0 | 0.516 | 0.001 | 38.8 | 0.0 | c | 23.50 | 0.487 | 0.003 | 12.3 | 0.0 |
| - | Mathilda Avenue/Ross Drive | E | City | Signal | C | 25.7 | 0.428 | 20.8 | c | 25.0 | 0.500 | 16.2 | c | 25.7 | 0.429 | 0.001 | 20.8 | 0.0 | c | 24.90 | 0.503 | 0.003 | 16.2 | 0.0 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | c | 26.2 | 0.574 | 24.9 | C- | 32.2 | 0.555 | 25.4 | c | 26.2 | 0.575 | 0.001 | 24.9 | 0.0 | C- | 32.00 | 0.559 | 0.004 | 25.3 | -0.1 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | A | 8.3 | 0.648 | 9.5 | B | 13.3 | 0.464 | 19.2 | A | 8.2 | 0.620 | 0.002 | 9.5 | 0.0 | B | 13.20 | 0.471 | 0.007 | 19.0 | -0.2 |
| 6 | Mathilda Avenue/Maude Avenue | E | City | Signal | D | 49.1 | 0.828 | 67.7 | D- | 55.0 | 0.800 | 57.1 | D | 49.1 | 0.828 | 0.000 | 67.7 | 0.0 | E+ | 55.10 | 0.804 | 0.004 | 57.2 | 0.1 |
| 7 | Mathilda Avenue/lndio Way | E | City | Signal | D+ | 38.7 | 0.771 | 39.2 | C- | 34.1 | 0.856 | 37.8 | D+ | 38.6 | 0.774 | 0.003 | 39.3 | 0.1 | C- | 34.40 | 0.864 | 0.008 | 38.4 | 0.6 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | c | 27.4 | 0.647 | 20.3 | C- | 34.7 | 0.840 | 34.8 | c | 27.3 | 0.650 | 0.003 | 20.3 | 0.0 | D+ | 35.00 | 0.849 | 0.009 | 35.4 | 0.6 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | D+ | 38.3 | 0.827 | 40.2 | D+ | 38.4 | 0.810 | 34.2 | D+ | 38.1 | 0.830 | 0.003 | 40.3 | 0.1 | D+ | 38.40 | 0.818 | 0.008 | 34.4 | 0.2 |
| 10 | Mathilda Avenue/Mckinley Avenue | E | City | Signal | B | 15.0 | 0.668 | 14.9 | B- | 18.2 | 0.619 | 17.0 | B | 15.3 | 0.673 | 0.005 | 15.3 | 0.4 | B- | 18.30 | 0.630 | 0.011 | 17.3 | 0.3 |
| 11 | Mathilda Avenuelowa Avenue | E | City | Signal | B | 17.9 | 0.622 | 13.6 | B- | 18.3 | 0.533 | 11.8 | B- | 19.5 | 0.632 | 0.010 | 15.2 | 1.6 | C+ | 22.60 | 0.589 | 0.056 | 17.1 | 5.3 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B | 16.7 | 0.624 | 11.2 | C+ | 20.4 | 0.587 | 15.3 | B | 16.9 | 0.633 | 0.009 | 11.5 | 0.3 | C+ | 21.30 | 0.609 | 0.022 | 16.9 | 1.6 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Caltrans | Signal | E+ | 57.1 | 0.796 | 59.4 | E+ | 57.3 | 0.836 | 61.4 | E+ | 57.7 | 0.806 | 0.010 | 60.2 | 0.8 | E+ | 57.20 | 0.835 | $-0.001$ | 61.1 | -0.3 |
| 14 | Sunnyvale Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.1 | 0.427 | 36.6 | D | 48.9 | 0.712 | 52.5 | D | 42.0 | 0.428 | 0.001 | 36.5 | -0.1 | D | 48.80 | 0.714 | 0.002 | 52.5 | 0.0 |
| 15 | Mathilda Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | c | 26.8 | 0.583 | 26.1 | C- | 33.3 | 0.569 | 32.7 | c | 26.7 | 0.587 | 0.004 | 26.0 | -0.1 | C- | 33.10 | 0.576 | 0.007 | 32.6 | -0.1 |
| 16 | Sunnyval-Saratoga Road/Remington Drive | E | City | Signal | D | 46.3 | 0.848 | 42.6 | D | 49.3 | 0.790 | 51.1 | D | 46.3 | 0.853 | 0.005 | 42.7 | 0.1 | D | 49.30 | 0.797 | 0.007 | 51.2 | 0.1 |
| 17 | Sunnyval-Saratoga Road/Fremont Avenue | E | City | Signal | D. | 52.9 | 0.838 | 52.2 | D. | 53.5 | 0.806 | 52.4 | D. | 52.9 | 0.843 | 0.005 | 52.3 | 0.1 | D. | 53.60 | 0.813 | 0.007 | 52.5 | 0.1 |
| 18 | S Pastoria Avenue $W$ Washington Avenue | D | City | Signal | B | 13.6 | 0.333 | 13.9 | B | 14.2 | 0.411 | 14.6 | B | 13.6 | 0.333 | 0.000 | 13.9 | 0.0 | B | 14.20 | 0.411 | 0.000 | 14.6 | 0.0 |
| 19 | S Pastoria AvenueN lowa Avenue | D | City | Signal | A | 9.1 | 0.280 | 9.2 | B+ | 10.6 | 0.346 | 10.6 | A | 9.1 | 0.280 | 0.000 | 9.2 | 0.0 | B+ | 10.60 | 0.346 | 0.000 | 10.6 | 0.0 |
| 20 | S Pastoria AvenueN Olive Avenue | D | City | ASWC | B | 10.9 | 0.372 | 10.9 | B | 12.5 | 0.434 | 12.5 | B | 10.9 | 0.374 | 0.002 | 10.9 | 0.0 | B | 12.70 | 0.440 | 0.006 | 12.7 | 0.2 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.2 | 0.524 | 38.7 | D | 42.3 | 0.628 | 43.0 | D | 42.4 | 0.525 | 0.001 | 39.0 | 0.3 | D | 42.60 | 0.630 | 0.002 | 43.2 | 0.2 |
| 22 | Charles StreetW lowa Avenue | D | City | sssc | B | 10.1 | 0.023 | 2.5 | B | 11.2 | 0.043 | 2.5 | B | 11.4 | 0.055 | 0.032 | 4.3 | 1.8 | B | 14.10 | 0.130 | 0.087 | 5.0 | 2.5 |
| 23 | Mary Avenue/W Olive Avenue | D | City | SSSC | F | 77.3 | 0.307 | 4.4 | F | 89.3 | 0.394 | 4.6 | F | 84.1 | 0.343 | 0.036 | 4.7 | 0.3 | F | 92.80 | 0.400 | 0.006 | 5.5 | 0.9 |
| 24 | Mary Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 50.1 | 0.732 | 50.3 | D- | 52.2 | 0.784 | 51.6 | D | 50.2 | 0.737 | 0.005 | 50.6 | 0.3 | D. | 52.80 | 0.794 | 0.010 | 52.4 | 0.8 |
| 25 | Sunnyval-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | B | 12.7 | 0.600 | 10.8 | B+ | 10.4 | 0.521 | 9.6 | B | 12.6 | 0.605 | 0.005 | 10.8 | 0.0 | B+ | 10.40 | 0.528 | 0.007 | 9.5 | -0.1 |
| 26 | Sunnyval-Saratoga Road/Alberta Avenue-Harwick Way | E | City | Signal | c | 26.4 | 0.671 | 23.0 | c | 25.7 | 0.612 | 25.4 | c | 26.3 | 0.675 | 0.004 | 23.0 | 0.0 | c | 25.60 | 0.618 | 0.006 | 25.3 | -0.1 |
| 27 | Sunnyval-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | D | 47.8 | 0.930 | 53.0 | D | 50.4 | 0.924 | 57.3 | D | 48.2 | 0.934 | 0.004 | 53.5 | 0.5 | D | 51.00 | 0.931 | 0.007 | 58.1 | 0.8 |
| Note: | The average control delay is reported for signalized and AWSC int Locations operating unacceptably are in bold and impacts are high |  | for the w | movement | eported | for SSSC | sec |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## Kimley»Horn

All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (AM and PM peak hours)

This is a significant impact in the AM peak hour because the intersection operates at an unacceptable LOS F and the project increases the v/c by more than 0.01.

## Existing Plus Background Plus Project - Option 2

Existing Plus Background Plus Project (Option 2) volumes were generated by adding the project trips to the Existing Plus Background volumes. Table 9 shows the Existing Plus Background Plus Project LOS and delay for each of the study intersections. The LOS calculations are attached.

All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (AM and PM peak hours)

This is a significant impact in the AM peak hour because the intersection operates at an unacceptable LOS F and the project increases the v/c by more than 0.01 .

Table 9 - Existing Plus Background Plus Project (Option 2) LOS Summary

| \# | Intersection | $\underset{\text { LOS }}{\text { LOTeria }}$ | Jurisdiction | Control | Background |  |  |  |  |  |  |  | Background + Project (Option 2) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AM Peak |  |  |  | PMPeak |  |  |  | AM Peak |  |  |  |  |  | PM Peak |  |  |  |  |  |
|  |  |  |  |  | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{\prime} \end{aligned}$ | vc ratio | $\begin{aligned} & \text { Crit. } \\ & \text { Delay } \end{aligned}$ | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | $\begin{gathered} c_{\text {Crit. }}^{\substack{\text { Delay }}} . \end{gathered}$ | LOS | $\begin{aligned} & \text { Deay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/cratio | Var | $\begin{array}{\|l\|l\|} \hline \text { Cintical } \\ \text { Delay } \\ \text { (sean) } \end{array}$ | Var | LOS | $\begin{aligned} & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | vc ratio | Var | $\begin{gathered} \text { Critical } \\ \text { Delay } \\ (\mathrm{sec}) \\ \hline \end{gathered}$ | Var |
| 1 | Mathilda Avenue-WB SR-237 Ramps | E | City | Signal | A | 0.00 | 0.000 | 0 | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.000 | 0.0 | 0.0 | A | 0.0 | 0.000 | 0.000 | 0.0 | 0.0 |
| 2 | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | D+ | 37.0 | 0.515 | 38.8 | c | 23.5 | 0.484 | 12.3 | D+ | 37.0 | 0.516 | 0.001 | 38.8 | 0.0 | c | 23.5 | 0.487 | 0.003 | 12.3 | 0.0 |
| 3 | Mathilda Avenue/Ross Drive | E | City | Signal | c | 25.7 | 0.428 | 20.8 | c | 25.0 | 0.500 | 16.2 | c | 25.7 | 0.429 | 0.001 | 20.8 | 0.0 | c | 24.9 | 0.503 | 0.003 | 16.2 | 0.0 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | c | 26.2 | 0.574 | 24.9 | C- | 32.2 | 0.555 | 25.4 | c | 26.2 | 0.575 | 0.001 | 24.9 | 0.0 | c- | 32.0 | 0.559 | 0.004 | 25.3 | -0.1 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | A | 8.3 | 0.618 | 9.5 | B | 13.3 | 0.464 | 19.2 | A | 8.2 | 0.620 | 0.002 | 9.5 | 0.0 | B | 13.2 | 0.471 | 0.007 | 19.0 | -0.2 |
| 6 | Mathilda Avenue/Maude Avenue | E | City | Signal | D | 49.1 | 0.828 | 67.7 | D- | 55.0 | 0.800 | 57.1 | D | 49.1 | 0.828 | 0.000 | 67.7 | 0.0 | E+ | 55.1 | 0.804 | 0.004 | 57.2 | 0.1 |
| 7 | Mathilda Avenue/ndio Way | E | City | Signal | ${ }^{\text {D }+}$ | 38.7 | 0.771 | 39.2 | C- | 34.1 | 0.856 | 37.8 | D+ | 38.7 | 0.774 | 0.003 | 39.3 | 0.1 | C- | 34.5 | 0.864 | 0.008 | 38.5 | 0.7 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | c | 27.4 | 0.647 | 20.3 | C- | 34.7 | 0.840 | 34.8 | c | 27.3 | 0.650 | 0.003 | 20.3 | 0.0 | D+ | 35.0 | 0.849 | 0.009 | 35.4 | 0.6 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | D+ | 38.3 | 0.827 | 40.2 | D+ | 38.4 | 0.810 | 34.2 | D+ | 38.1 | 0.830 | 0.003 | 40.3 | 0.1 | D+ | 38.4 | 0.818 | 0.008 | 34.4 | 0.2 |
| 10 | Mathilda AvenueMMCKinley Avenue | E | City | Signal | B | 15.0 | 0.668 | 14.9 | B- | 18.2 | 0.619 | 17.0 | B | 15.2 | 0.673 | 0.005 | 15.2 | 0.3 | B- | 18.8 | 0.635 | 0.016 | 17.9 | 0.9 |
| 11 | Mathilda Avenue/lowa Avenue | E | City | Signal | B | 17.9 | 0.622 | 13.6 | B- | 18.3 | 0.533 | 11.8 | B- | 19.0 | 0.632 | 0.010 | 15.2 | 1.6 | C+ | 22.8 | 0.589 | 0.056 | 18.0 | 6.2 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B | 16.7 | 0.624 | 11.2 | C+ | 20.4 | 0.587 | 15.3 | C+ | 20.8 | 0.660 | 0.036 | 14.3 | 3.1 | C+ | 22.6 | 0.611 | 0.024 | 18.3 | 3.0 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Catrrans | Signal | E+ | 57.1 | 0.796 | 59.4 | E+ | 57.3 | 0.836 | 61.4 | E+ | 58.7 | 0.835 | 0.039 | 62.2 | 2.8 | E+ | 59.4 | 0.873 | 0.037 | 64.2 | 2.8 |
| 14 | Sunnyvale Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.1 | 0.427 | 36.6 | D | 48.9 | 0.712 | 52.5 | D | 42.0 | 0.428 | 0.001 | 36.5 | -0.1 | D | 48.8 | 0.714 | 0.002 | 52.5 | 0.0 |
| 15 | Mathilda Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | c | 26.8 | 0.583 | 26.1 | C- | 33.3 | 0.569 | 32.7 | c | 26.7 | 0.586 | 0.003 | 26.1 | 0.0 | C- | 33.1 | 0.574 | 0.005 | 32.6 | -0.1 |
| 16 | Sunnyval-Saratoga Road/Remington Drive | E | City | Signal | D | 46.3 | 0.848 | 42.6 | D | 49.3 | 0.790 | 51.1 | D | 46.3 | 0.852 | 0.004 | 42.7 | 0.1 | D | 49.3 | 0.795 | 0.005 | 51.2 | 0.1 |
| 17 | Sunnyval-SSaratoga Road/Fremont Avenue | E | City | Signal | D. | 52.9 | 0.838 | 52.2 | D. | 53.5 | 0.806 | 52.4 | D- | 52.9 | 0.842 | 0.004 | 52.3 | 0.1 | D. | 53.6 | 0.811 | 0.005 | 52.5 | 0.1 |
| 18 | S Pastoria AvenueW Washington Avenue | D | City | Signal | B | 13.6 | 0.333 | 13.9 | B | 14.2 | 0.411 | 14.6 | B | 13.6 | 0.333 | 0.000 | 13.9 | 0.0 | B | 14.2 | 0.411 | 0.000 | 14.6 | 0.0 |
| 19 | S Pastoria AvenueN lowa Avenue | D | City | Signal | A | 9.1 | 0.280 | 9.2 | B+ | 10.6 | 0.346 | 10.6 | A | 9.6 | 0.281 | 0.001 | 9.9 | 0.7 | B+ | 11.4 | 0.374 | 0.028 | 12.0 | 1.4 |
| 20 | S Pastoria AvenueN Olive Avenue | D | City | ASWC | B | 10.9 | 0.372 | 10.9 | B | 12.5 | 0.434 | 12.5 | B | 10.4 | 0.359 | -0.013 | 10.4 | $-0.5$ | B | 11.2 | 0.423 | ${ }^{-0.011}$ | 11.2 | -1.3 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.2 | 0.524 | 38.7 | D | 42.3 | 0.628 | 43.0 | D | 46.0 | 0.600 | 0.076 | 44.6 | 5.9 | D | 46.1 | 0.705 | 0.077 | 48.7 | 5.7 |
| 22 | Charles StreetW lowa Avenue | D | City | Sssc | B | 10.1 | 0.023 | 2.5 | B | 11.2 | 0.043 | 2.5 | B | 10.7 | 0.033 | 0.010 | 2.9 | 0.4 | B | 12.7 | 0.059 | 0.016 | 2.8 | 0.3 |
| 23 | Mary Avenue/ Olive Avenue | D | City | SSSC | F | 77.3 | 0.307 | 4.4 | F | 89.3 | 0.394 | 4.6 | F | 84.1 | 0.343 | 0.036 | 4.7 | 0.3 | F | 92.8 | 0.400 | 0.006 | 5.5 | 0.9 |
| 24 | Mary Avenue/EI Camino Real | E | City / Catrrans | Signal | D | 50.1 | 0.732 | 50.3 | D. | 52.2 | 0.784 | 51.6 | D | 50.2 | 0.737 | 0.005 | 50.6 | 0.3 | D. | 52.8 | 0.794 | 0.010 | 52.4 | 0.8 |
| 25 | Sunnyval-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | B | 12.7 | 0.600 | 10.8 | B+ | 10.4 | 0.521 | 9.6 | B | 12.6 | 0.604 | 0.004 | 10.8 | 0.0 | B+ | 10.4 | 0.526 | 0.005 | 9.5 | -0.1 |
| 26 | Sunnyval--Saratoga Road/Alberta Avenue-Harwick Way | E | City | Signal | c | 26.4 | 0.671 | 23.0 | c | 25.7 | 0.612 | 25.4 | c | 26.4 | 0.674 | 0.003 | 23.0 | 0.0 | C | 25.6 | 0.617 | 0.005 | 25.3 | -0.1 |
| 27 | Sunnyval-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | D | 47.8 | 0.930 | 53.0 | D | 50.4 | 0.924 | 57.3 | D | 48.1 | 0.933 | 0.003 | 53.4 | 0.4 | D | 50.8 | 0.929 | 0.005 | 57.9 | 0.6 |
|  | The average control delay is reported for signalized and AWSC int Locations operating unacceptably are in bold and impacts are high | ons. Th | for the w | ement | reported | or SSSC | intersectio |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## Kimley»"Horn

## CUMULATIVE (2025) CONDITIONS

To achieve Cumulative (2025) traffic conditions, traffic volumes from approved and pending projects were incorporated according to the information provided by the City (dated September 2017). In addition, a 1.5 percent annual growth rate was added to the existing volumes. Table 10 shows the Cumulative (Future) LOS and delay for each of the study intersections. The LOS calculations are attached.

Table 10 - Cumulative (2025) LOS Summary

| \# | Intersection | $\begin{aligned} & \text { LOS } \\ & \text { Criteria } \end{aligned}$ | Jurisdiction | Control | Cumulative (2025) |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AMPeak |  |  |  | PM Peak |  |  |  |
|  |  |  |  |  | LOS | $\begin{gathered} \text { Delay } \\ (\mathrm{sec})^{1} \end{gathered}$ | v/c ratio | Crit. <br> Delay | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Crit. <br> Delay |
| 1 | Mathilda Avenue/WB SR-237 Ramps | E | City | Signal | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.0 |
| 2 | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | D+ | 38.4 | 0.575 | 40.3 | C | 24.7 | 0.546 | 13.6 |
| 3 | Mathilda Avenue/Ross Drive | E | City | Signal | C | 26.4 | 0.473 | 21.1 | C | 26.1 | 0.561 | 17.3 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | C | 27.4 | 0.646 | 26.6 | C- | 33.1 | 0.618 | 26.4 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | B | 13.9 | 0.734 | 16.7 | C | 23.3 | 0.628 | 31.7 |
| 6 | Mathilda Avenue/Maude Avenue | E | City | Signal | E+ | 56.5 | 0.929 | 80.4 | E | 61.1 | 0.904 | 65.1 |
| 7 | Mathilda Avenue/Indio Way | E | City | Signal | D | 42.9 | 0.860 | 45.9 | D | 43.4 | 0.969 | 51.2 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | C | 30.3 | 0.732 | 23.9 | D | 42.9 | 0.955 | 47.3 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | D | 46.0 | 0.935 | 50.3 | D | 46.7 | 0.922 | 45.8 |
| 10 | Mathilda Avenue/McKinley Avenue | E | City | Signal | B- | 18.5 | 0.795 | 19.8 | C+ | 21.0 | 0.713 | 20.4 |
| 11 | Mathilda Avenue/lowa Avenue | E | City | Signal | B- | 19.3 | 0.705 | 15.6 | B- | 19.4 | 0.613 | 16.2 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B- | 18.1 | 0.707 | 12.8 | C+ | 21.3 | 0.663 | 16.5 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Caltrans | Signal | E | 62.3 | 0.894 | 66.3 | E | 64.9 | 0.949 | 73.3 |
| 14 | Sunnyvale Avenue/El Camino Real | E | City / Caltrans | Signal | D | 42.0 | 0.467 | 35.6 | D- | 52.1 | 0.806 | 56.8 |
| 15 | Mathilda Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | C | 27.1 | 0.649 | 26.7 | D+ | 35.4 | 0.645 | 35.0 |
| 16 | Sunnyvale-Saratoga Road/Remington Drive | E | City | Signal | E+ | 55.6 | 0.959 | 55.6 | E+ | 55.9 | 0.893 | 58.7 |
| 17 | Sunnyvale-Saratoga Road/Fremont Avenue | E | City | Signal | E | 60.1 | 0.944 | 63.2 | E+ | 59.2 | 0.907 | 60.0 |
| 18 | S Pastoria Avenue/W Washington Avenue | D | City | Signal | B | 13.9 | 0.382 | 14.3 | B | 14.8 | 0.476 | 15.2 |
| 19 | S Pastoria Avenue/W lowa Avenue | D | City | Signal | A | 9.3 | 0.319 | 9.5 | B+ | 10.8 | 0.404 | 10.9 |
| 20 | S Pastoria Avenue/W Olive Avenue | D | City | ASWC | B | 12.1 | 0.451 | 12.1 | B | 14.4 | 0.526 | 14.4 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 43.3 | 0.592 | 40.2 | D | 44.1 | 0.715 | 45.8 |
| 22 | Charles Street/W lowa Avenue | D | City | SSSC | B | 10.4 | 0.027 | 2.4 | B | 11.7 | 0.051 | 2.6 |
| 23 | Mary Avenue/W Olive Avenue | D | City | SSSC | F | 206.6 | 0.574 | 10.1 | F | 274.7 | 0.711 | 13.8 |
| 24 | Mary Avenue/EI Camino Real | E | City / Caltrans | Signal | D- | 53.4 | 0.828 | 55.0 | E+ | 57.7 | 0.888 | 58.9 |
| 25 | Sunnyvale-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | B | 13.1 | 0.673 | 11.6 | B+ | 10.8 | 0.585 | 10.1 |
| 26 | Sunnyvale-Saratoga Road/Alberta Avenue-Harwick Way | E | City | Signal | C | 28.1 | 0.752 | 25.3 | C | 27.3 | 0.687 | 27.5 |
| 27 | Sunnyvale-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | E | 62.5 | 1.036 | 77.1 | E | 65.9 | 1.030 | 80.8 |

Note: Locations operating unacceptably are in bold.
All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (AM and PM peak hours)


## CUMULATIVE (2025) PLUS PROJECT CONDITIONS

## Cumulative (2025) Plus Project - Option 1

Cumulative (2025) Plus Project (Option 1) volumes were generated by adding the project trips to the Cumulative (2025) volumes. Table 11 shows the Cumulative (2025) Plus Project LOS and delay for each of the study intersections. The LOS calculations are attached.

Table 11 - Cumulative (2025) Plus Project (Option 1) LOS Summary

|  | Intersection | $\begin{aligned} & \text { LOS } \\ & \text { Criteria } \end{aligned}$ | Jurisdiction | Control | Cumulative (2025) |  |  |  |  |  |  |  | Cumulative (2025) + Project (Option 1) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AMPeak |  |  |  | PMPeak |  |  |  | AM Peak |  |  |  |  |  | PM Peak |  |  |  |  |  |
| \# |  |  |  |  | LOS | $\begin{aligned} & \text { Delay } \\ & (\text { (sec })^{1} \end{aligned}$ | V/c ratio | $\begin{aligned} & \text { Crit. } \\ & \text { Delay } \end{aligned}$ | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | vc ratio | $\begin{gathered} \text { Crit. } \\ \text { Delay } \end{gathered}$ | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Var | $\begin{aligned} & \text { Critical } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Var | LOS | $\begin{gathered} \text { Delay } \\ (\mathrm{sec})^{1} \end{gathered}$ | vc ratio | Var | $\begin{gathered} \text { Critical } \\ \text { Delay } \\ \text { (sece } \end{gathered}$ | Var |
| 1 | Mathilda Avenue/WB SR-237 Ramps | E | City | Signal | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.000 | 0.0 | 0.0 | A | 0.0 | 0.000 | 0.000 | 0.0 | 0.0 |
| 2 | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | D+ | 38.4 | 0.575 | 40.3 | c | 24.7 | 0.546 | 13.6 | D+ | 38.4 | 0.575 | 0.000 | 40.3 | 0.0 | c | 24.8 | 0.549 | 0.003 | 13.5 | -0.1 |
| 3 | Mathilda Avenue/Ross Drive | E | City | Signal | C | 26.4 | 0.473 | 21.1 | c | 26.1 | 0.561 | 17.3 | C | 26.4 | 0.474 | 0.001 | 21.1 | 0.0 | c | 26.1 | 0.564 | 0.003 | 17.3 | 0.0 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | c | 27.4 | 0.646 | 26.6 | C- | 33.1 | 0.618 | 26.4 | c | 27.4 | 0.648 | 0.002 | 26.6 | 0.0 | C- | 33.0 | 0.622 | 0.004 | 26.3 | -0.1 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | B | 13.9 | 0.734 | 16.7 | c | 23.3 | 0.628 | 31.7 | B | 13.8 | 0.736 | 0.002 | 16.7 | 0.0 | c | 23.1 | 0.636 | 0.008 | 31.5 | -0.2 |
| 6 | Mathilda Avenue/Maude Avenue | E | City | Signal | E+ | 56.5 | 0.929 | 80.4 | E | 61.1 | 0.904 | 65.1 | E+ | 56.5 | 0.929 | 0.000 | 80.4 | 0.0 | E | 61.4 | 0.908 | 0.004 | 65.4 | 0.3 |
| 7 | Mathilda Avenue/Indio Way | E | City | Signal | D | 42.9 | 0.860 | 45.9 | D | 43.4 | 0.969 | 51.2 | D | 43.0 | 0.862 | 0.002 | 46.0 | 0.1 | D | 44.5 | 0.977 | 0.008 | 53.0 | 1.8 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | C | 30.3 | 0.732 | 23.9 | D | 42.9 | 0.955 | 47.3 | c | 30.2 | 0.735 | 0.003 | 23.9 | 0.0 | D | 43.9 | 0.965 | 0.010 | 49.0 | 1.7 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | D | 46.0 | 0.935 | 50.3 | D | 46.7 | 0.922 | 45.8 | D | 46.0 | 0.938 | 0.003 | 50.6 | 0.3 | D | 47.2 | 0.929 | 0.007 | 46.8 | 1.0 |
| 10 | Mathilda Avenue/McKinlley Avenue | E | City | Signal | B- | 18.5 | 0.795 | 19.8 | C+ | 21.0 | 0.713 | 20.4 | B- | 18.8 | 0.800 | 0.005 | 20.3 | 0.5 | ${ }_{\text {C+ }}$ | 21.2 | 0.723 | 0.010 | 20.9 | 0.5 |
| 11 | Mathilda Avenue/lowa Avenue | E | City | Signal | B- | 19.3 | 0.705 | 15.6 | B- | 19.4 | 0.613 | 16.2 | C+ | 20.9 | 0.715 | 0.010 | 17.2 | 1.6 | c | 23.3 | 0.659 | 0.046 | 18.3 | 2.1 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B- | 18.1 | 0.707 | 12.8 | ${ }^{\text {C+ }}$ | 21.3 | 0.663 | 16.5 | B- | 18.1 | 0.713 | 0.006 | 12.7 | -0.1 | ${ }^{+}+$ | 22.5 | 0.688 | 0.025 | 18.7 | 2.2 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Caltrans | Signal | E | 62.3 | 0.894 | 66.3 | E | 64.9 | 0.949 | 73.3 | E | 63.3 | 0.904 | 0.010 | 67.7 | 1.4 | E | 64.7 | 0.949 | 0.000 | 73.0 | -0.3 |
| 14 | Sunnyval Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.0 | 0.467 | 35.6 | D- | 52.1 | 0.806 | 56.8 | D | 42.0 | 0.468 | 0.001 | 35.5 | $-0.1$ | D- | 52.1 | 0.808 | 0.002 | 56.8 | 0.0 |
| 15 | Mathilda Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | c | 27.1 | 0.649 | 26.7 | D+ | 35.4 | 0.645 | 35.0 | c | 27.0 | 0.654 | 0.005 | 26.7 | 0.0 | D+ | 35.3 | 0.651 | 0.006 | 35.0 | 0.0 |
| 16 | Sunnyval-Saratoga Road/Remington Drive | E | City | Signal | E+ | 55.6 | 0.959 | 55.6 | E+ | 55.9 | 0.893 | 58.7 | E+ | 56.1 | 0.963 | 0.004 | 56.3 | 0.7 | E+ | 56.1 | 0.899 | 0.006 | 59.1 | 0.4 |
| 17 | Sunnyval-Saratoga Road/Fremont Avenue | E | City | Signal | E | 60.1 | 0.944 | 63.2 | E+ | 59.2 | 0.907 | 60.0 | E | 60.4 | 0.948 | 0.004 | 63.7 | 0.5 | E+ | 59.5 | 0.913 | 0.006 | 60.5 | 0.5 |
| 18 | S Pastoria Avenue/W Washington Avenue | D | City | Signal | B | 13.9 | 0.382 | 14.3 | B | 14.8 | 0.476 | 15.2 | B | 13.9 | 0.382 | 0.000 | 14.3 | 0.0 | B | 14.8 | 0.476 | 0.000 | 15.2 | 0.0 |
| 19 | S Pastoria Avenue/W lowa Avenue | D | City | Signal | A | 9.3 | 0.319 | 9.5 | B+ | 10.8 | 0.404 | 10.9 | A | 9.3 | 0.319 | 0.000 | 9.5 | 0.0 | B+ | 10.8 | 0.404 | 0.000 | 10.9 | 0.0 |
| 20 | S Pastoria Avenue/W Olive Avenue | D | City | ASWC | B | 12.1 | 0.451 | 12.1 | B | 14.4 | 0.526 | 14.4 | B | 12.1 | 0.453 | 0.002 | 12.1 | 0.0 | B | 14.6 | 0.532 | 0.006 | 14.6 | 0.2 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 43.3 | 0.592 | 40.2 | D | 44.1 | 0.715 | 45.8 | D | 43.4 | 0.593 | 0.001 | 40.4 | 0.2 | D | 44.5 | 0.717 | 0.002 | 46.0 | 0.2 |
| 22 | Charles StreetW lowa Avenue | D | City | sssc | B | 10.4 | 0.027 | 2.4 | B | 11.7 | 0.051 | 2.6 | B | 11.7 | 0.058 | 0.031 | 4.2 | 1.8 | c | 15.0 | 0.136 | 0.085 | 5.0 | 2.4 |
| 23 | Mary Avenue/W Olive Avenue | D | City | SSsC | F | 206.6 | 0.574 | 10.1 | F | 274.7 | 0.711 | 13.8 | F | 23.0 | 0.635 | 0.061 | 11.1 | 1.0 | F | 389.0 | 0.829 | 0.118 | 18.7 | 4.9 |
| 24 | Mary Avenue/EI Camino Real | E | City / Caltrans | Signal | D- | 53.4 | 0.828 | 55.0 | E+ | 57.7 | 0.888 | 58.9 | D- | 53.6 | 0.831 | 0.003 | 55.3 | 0.3 | E+ | 58.7 | 0.898 | 0.010 | 60.2 | 1.3 |
| 25 | Sunnyale-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | B | 13.1 | 0.673 | 11.6 | B+ | 10.8 | 0.585 | 10.1 | B | 13.1 | 0.677 | 0.004 | 11.6 | 0.0 | B+ | 10.8 | 0.591 | 0.006 | 10.1 | 0.0 |
| 26 | Sunnyval-Saratoga Road/Alberta Avenue-Harwick Way | E | City | Signal | c | 28.1 | 0.752 | 25.3 | C | 27.3 | 0.687 | 27.5 | c | 28.1 | 0.756 | 0.004 | 25.3 | 0.0 | C | 27.2 | 0.694 | 0.007 | 27.4 | -0.1 |
| 27 | Sunnyval-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | E | 62.5 | 1.036 | 77.1 | E | 65.9 | 1.030 | 80.8 | E | 63.3 | 1.041 | 0.005 | 78.4 | 1.3 | E | 67.1 | 1.037 | 0.007 | 82.6 | 1.8 |

Note: Locations operating unacceptably are in bold and impacts are highlighted.

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All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (AM and PM peak hours)

This is a significant impact in the AM and PM peak hours because the intersection operates at an unacceptable LOS F and the project increases the v/c by more than 0.01.

Cumulative (2025) Plus Project - Option 2
Cumulative (2025) Plus Project (Option 2) volumes were generated by adding the project trips to the Cumulative (2025) volumes. Table 12 shows the Cumulative (2025) Plus Project LOS and delay for each of the study intersections. The LOS calculations are attached.

All study intersections meet the acceptable LOS requirements during the AM and PM peak hours except for:

- Int \#23 - Mary Avenue/W Olive Avenue (AM and PM peak hours)

This is a significant impact in the AM and PM peak hours because the intersection operates at an unacceptable LOS F and the project increases the v/c by more than 0.01.

Table 12 - Cumulative (2025) Plus Project (Option 2) LOS Summary

| \# | Intersection | LOSCriteria | Jurisdiction | Control | Cumulative (2025) |  |  |  |  |  |  |  | Cumulative (2025) + Project (Option 2) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | AM Peak |  |  |  | PMPeak |  |  |  | AM Peak |  |  |  |  |  | PM Peak |  |  |  |  |  |
|  |  |  |  |  | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | V/c ratio | Crit. Delay | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | $\begin{aligned} & \text { Crit. } \\ & \text { Delay } \end{aligned}$ | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Var | $\begin{aligned} & \text { Critical } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Var | LOS | $\begin{aligned} & \text { Delay } \\ & (\mathrm{sec})^{1} \end{aligned}$ | v/c ratio | Var | $\begin{aligned} & \text { Critical } \\ & \text { Delay } \\ & \text { (sec) } \end{aligned}$ | Var |
| 1 | Mathilda Avenue/WB SR-237 Ramps | E | City | Signal | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.0 | A | 0.0 | 0.000 | 0.000 | 0.0 | 0.0 | A | 0.0 | 0.000 | 0.000 | 0.0 | 0.0 |
| 2 | Mathilda Avenue/EB SR-237 Ramps | E | City | Signal | D+ | 38.4 | 0.575 | 40.3 | c | 24.7 | 0.546 | 13.6 | D+ | 38.4 | 0.575 | 0.000 | 40.3 | 0.0 | c | 24.8 | 0.549 | 0.003 | 13.5 | -0.1 |
| 3 | Mathilda Avenue/Ross Drive | E | City | Signal | c | 26.4 | 0.473 | 21.1 | c | 26.1 | 0.561 | 17.3 | c | 26.4 | 0.474 | 0.001 | 21.1 | 0.0 | c | 26.1 | 0.564 | 0.003 | 17.3 | 0.0 |
| 4 | Mathilda Avenue/Ahwanee Avenue | E | City | Signal | c | 27.4 | 0.646 | 26.6 | C- | 33.1 | 0.618 | 26.4 | c | 27.4 | 0.648 | 0.002 | 26.6 | 0.0 | C- | 33.0 | 0.622 | 0.004 | 26.3 | -0.1 |
| 5 | Mathilda Avenue/San Aleso Avenue | E | City | Signal | B | 13.9 | 0.734 | 16.7 | c | 23.3 | 0.628 | 31.7 | B | 13.8 | 0.736 | 0.002 | 16.7 | 0.0 | c | 23.1 | 0.636 | 0.008 | 31.5 | -0.2 |
| 6 | Mathilda Avenue/Maude Avenue | E | City | Signal | E+ | 56.5 | 0.929 | 80.4 | E | 61.1 | 0.904 | 65.1 | E+ | 56.5 | 0.929 | 0.000 | 80.4 | 0.0 | E | 61.4 | 0.908 | 0.004 | 65.4 | 0.3 |
| 7 | Mathildd Avenue/Indio Way | E | City | Signal | D | 42.9 | 0.860 | 45.9 | D | 43.4 | 0.969 | 51.2 | D | 43.0 | 0.862 | 0.002 | 46.0 | 0.1 | D | 44.6 | 0.977 | 0.008 | 53.1 | 1.9 |
| 8 | Mathilda Avenue/California Avenue | E | City | Signal | C | 30.3 | 0.732 | 23.9 | D | 42.9 | 0.955 | 47.3 | C | 30.2 | 0.735 | 0.003 | 23.9 | 0.0 | D | 43.9 | 0.965 | 0.010 | 49.0 | 1.7 |
| 9 | Mathilda Avenue/Washington Avenue | E | City | Signal | D | 46.0 | 0.935 | 50.3 | D | 46.7 | 0.922 | 45.8 | D | 46.0 | 0.938 | 0.003 | 50.6 | 0.3 | D | 47.2 | 0.929 | 0.007 | 46.8 | 1.0 |
| 10 | Mathilda Avenue/McKinley Avenue | E | City | Signal | B- | 18.5 | 0.795 | 19.8 | C+ | 21.0 | 0.713 | 20.4 | B- | 18.7 | 0.799 | 0.004 | 20.2 | 0.4 | C+ | 21.7 | 0.728 | 0.015 | 21.5 | 1.1 |
| 11 | Mathilda Avenue/lowa Avenue | E | City | Signal | B- | 19.3 | 0.705 | 15.6 | B- | 19.4 | 0.613 | 16.2 | C+ | 20.5 | 0.715 | 0.010 | 17.2 | 1.6 | c | 23.5 | 0.660 | 0.047 | 19.2 | 3.0 |
| 12 | Mathilda Avenue/Olive Avenue | E | City | Signal | B- | 18.1 | 0.707 | 12.8 | C+ | 21.3 | 0.663 | 16.5 | ${ }^{+}+$ | 22.1 | 0.745 | 0.038 | 16.1 | 3.3 | c | 25.2 | 0.705 | 0.042 | 22.0 | 5.5 |
| 13 | Mathilda Avenue/EI Camino Real | E | City / Caltrans | Signal | E | 62.3 | 0.894 | 66.3 | E | 64.9 | 0.949 | 73.3 | E | 66.1 | 0.939 | 0.045 | 72.5 | 6.2 | E | 72.2 | 0.999 | 0.050 | 84.9 | 11.6 |
| 14 | Sunnyvale Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 42.0 | 0.467 | 35.6 | D- | 52.1 | 0.806 | 56.8 | D | 42.0 | 0.468 | 0.001 | 35.5 | $-0.1$ | D- | 52.1 | 0.808 | 0.002 | 56.8 | 0.0 |
| 15 | Mathilda Avenue/Sunnyvale-Saratoga Road-Talisman Drive | E | City / Caltrans | Signal | C | 27.1 | 0.649 | 26.7 | D+ | 35.4 | 0.645 | 35.0 | C | 27.0 | 0.653 | 0.004 | 26.7 | 0.0 | D+ | 35.3 | 0.650 | 0.005 | 35.0 | 0.0 |
| 16 | Sunnyval-Saratoga Road/Remington Drive | E | City | Signal | E+ | 55.6 | 0.959 | 55.6 | E+ | 55.9 | 0.893 | 58.7 | E+ | 56.0 | 0.962 | 0.003 | 56.1 | 0.5 | E+ | 56.1 | 0.898 | 0.005 | 59.0 | 0.3 |
| 17 | Sunnyval--Saratoga Road/Fremont Avenue | E | City | Signal | E | 60.1 | 0.944 | 63.2 | E+ | 59.2 | 0.907 | 60.0 | E | 60.3 | 0.947 | 0.003 | 63.6 | 0.4 | E+ | 59.4 | 0.912 | 0.005 | 60.4 | 0.4 |
| 18 | S Pastoria Avenue/W Washington Avenue | D | City | Signal | B | 13.9 | 0.382 | 14.3 | B | 14.8 | 0.476 | 15.2 | B | 13.9 | 0.382 | 0.000 | 14.3 | 0.0 | B | 14.8 | 0.476 | 0.000 | 15.2 | 0.0 |
| 19 | S Pastoria Avenue/W lowa Avenue | D | City | Signal | A | 9.3 | 0.319 | 9.5 | B+ | 10.8 | 0.404 | 10.9 | A | 9.7 | 0.320 | 0.001 | 10.1 | 0.6 | B+ | 11.6 | 0.430 | 0.026 | 12.1 | 1.2 |
| 20 | S Pastoria Avenue/W Olive Avenue | D | City | ASWC | B | 12.1 | 0.451 | 12.1 | B | 14.4 | 0.526 | 14.4 | B | 11.3 | 0.421 | ${ }^{-0.030}$ | 11.3 | -0.8 | B | 12.4 | 0.492 | -0.034 | 12.4 | -2.0 |
| 21 | Hollenbeck Avenue/EI Camino Real | E | City / Caltrans | Signal | D | 43.3 | 0.592 | 40.2 | D | 44.1 | 0.715 | 45.8 | D | 47.6 | 0.671 | 0.079 | 46.9 | 6.7 | D | 46.2 | 0.747 | 0.032 | 48.1 | 2.3 |
| 22 | Charles StreetW lowa Avenue | D | City | SSSC | B | 10.4 | 0.027 | 2.4 | B | 11.7 | 0.051 | 2.6 | B | 11.1 | 0.038 | 0.011 | 2.8 | 0.4 | B | 13.4 | 0.067 | 0.016 | 2.9 | 0.3 |
| 23 | Mary Avenue/W Olive Avenue | D | City | SSSC | F | 206.6 | 0.574 | 10.1 | F | 274.7 | 0.711 | 13.8 | F | 230.0 | 0.635 | 0.061 | 11.1 | 1.0 | F | 389.0 | 0.829 | 0.118 | 18.7 | 4.9 |
| 24 | Mary Avenue/EI Camino Real | E | City / Caltrans | Signal | D- | 53.4 | 0.828 | 55.0 | E+ | 57.7 | 0.888 | 58.9 | D- | 53.6 | 0.831 | 0.003 | 55.3 | 0.3 | E+ | 58.7 | 0.898 | 0.010 | 60.2 | 1.3 |
| 25 | Sunnyvale-Saratoga Road/Cheyenne Drive-Connemara Way | E | City | Signal | B | 13.1 | 0.673 | 11.6 | B+ | 10.8 | 0.585 | 10.1 | B | 13.1 | 0.676 | 0.003 | 11.6 | 0.0 | B+ | 10.8 | 0.590 | 0.005 | 10.1 | 0.0 |
| 26 | Sunnyval-Saratoga Road/Alberta Avenue-Harwick Way | E | City | Signal | c | 28.1 | 0.752 | 25.3 | c | 27.3 | 0.687 | 27.5 | c | 28.1 | 0.755 | 0.003 | 25.3 | 0.0 | c | 27.2 | 0.692 | 0.005 | 27.4 | -0.1 |
| 27 | Sunnyval-Saratoga Road-De Anza Boulevard/Homestead Road | E | City | Signal | E | 62.5 | 1.036 | 77.1 | E | 65.9 | 1.030 | 80.8 | , | 63.1 | 1.040 | 0.004 | 78.1 | 1.0 | , | 66.8 | 1.035 | 0.005 | 82.2 | 1.4 |

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## SUPPLEMENTAL SYNCHRO QUEUING EVALUATION

A Synchro evaluation for the intersections and driveways adjacent to the proposed Civic Center was conducted. This evaluation includes the intersections of Mathilda Avenue/Olive Avenue, Mathilda Avenue/El Camino Real, and Mathilda Avenue/Pastoria Avenue. The Synchro evaluation is better suited than Traffix to analyze closely spaced intersections and their interactions.

## Southbound Queues at Mathilda Avenue/EI Camino Real

There is particular interest along Mathilda Avenue in Option \#2 as many motorists will be exiting the mid-block driveway (All America Way) along Mathilda Avenue, just north of El Camino Real, and making a southbound U-turn movement to head northbound on Mathilda Avenue (due to the raised median on Mathilda Avenue). It should be noted that the LOS results for the Synchro model were checked against the Traffix model and they have similar results, but are not exactly the same. Since Synchro accounts for the distance between adjacent intersections and has more detailed input parameters for signal timing, the results will vary.

Table 13 shows the expected queues for the southbound approach at the intersection of Mathilda Avenue/El Camino Real.

Table 13 - Synchro Queuing Summary

| \# | Intersection | Scenario | Movement |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | SBL Queue (feet) |  | SBT Queue (feet) |  | SBR Queue (feet) |  |
|  |  |  | AM Peak | PM <br> Peak | AM Peak | PM <br> Peak | AM <br> Peak | PM <br> Peak |
| 13 | Mathilda <br> Avenue/ <br> El Camino Real | Storage Length | 435 |  | 855 |  | 315 |  |
|  |  | Existing Plus Project Option $\# 1$ | 174 | 492 | 139 | 841 | 209 | 160 |
|  |  | Existing Plus Project Option \#2 | 156 | 395 | 133 | 792 | 203 | 199 |
|  |  | Existing Plus Background Plus Project Option \#1 | 207 | 504 | 153 | 881 | 168 | 124 |
|  |  | Existing Plus Background Plus Project Option \#2 | 189 | 400 | 152 | 859 | 189 | 174 |
|  |  | Cumulative Plus Project Option \#1 | 231 | 490 | 167 | 919 | 188 | 119 |
|  |  | Cumulative Plus Project Option \#2 | 213 | 380 | 167 | 855 | 210 | 175 |

Note: The distance from the intersection of El Camino Real/Mathilda Avenue to All America Way is 315 feet.
Queue lengths shown in bold represent queues that exceed the available storage.

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The queues for the southbound approach show that it would be difficult for vehicles to exit the All America Way driveway and move over to the southbound left turn lane. The $95^{\text {th }}$ percentile southbound left turn queue at Mathilda Avenue/El Camino Real exceeds the 315-foot distance between All America Way and El Camino Real for all project scenarios in the PM peak hour. In addition, the $95^{\text {th }}$ percentile southbound through queue at Mathilda Avenue/El Camino Real extends at least 792 feet back from El Camino Real, which would block vehicles exiting the All America Way driveway from getting into the southbound left turn lanes. Therefore, it is recommended that the layout be adjusted to allow for outbound trips to go northbound on Mathilda Avenue.

In addition, a SimTraffic analysis was completed to confirm these conclusions regarding the southbound queues at the intersection of El Camino Real/Mathilda Avenue. The SimTraffic analysis showed that the 95th percentile southbound left turn queues will be 300 feet, the southbound through queues will be 343 feet, and the southbound right turn queues will be 321 feet for the Existing Plus Project Option \#2 PM peak hour. Since these queues would exceed the 315 feet between All America Way and El Camino Real, vehicles would not be able to exit the All America Way driveway and enter into the southbound left turn lane at Mathilda Avenue/El Camino Real. Since this scenario should have less volumes than the Existing Plus Background and Cumulative volumes, it is assumed that these same issues would occur in those scenarios.

## Westbound Queues at Pastoria Avenue/El Camino Real

There is also interest along El Camino Real in Option \#2 as motorists can exit the mid-block driveway along Mathilda Avenue, just east of the proposed southwest parking garage, and making a westbound U-turn movement to head eastbound on El Camino Real (due to the raised median on El Camino Real).

Table 14 shows the expected queues for the westbound approach at the intersection of Pastoria Avenue/El Camino Real.

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Table 14 - Synchro Queuing Summary

| \# | Intersection | Scenario | Movement |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | WBL Queue (feet) |  | WBT Queue (feet) |  |
|  |  |  | AM <br> Peak | PM <br> Peak | AM <br> Peak | $\begin{aligned} & \text { PM } \\ & \text { Peak } \end{aligned}$ |
| 21 | Pastoria <br> Avenue/ <br> El Camino <br> Real | Storage Length | 290 and 335 |  | 1,045 |  |
|  |  | Existing Plus Project Option \#1 | 106 | 272 | 526 | 375 |
|  |  | Existing Plus Project Option \#2 | 106 | 272 | 543 | 400 |
|  |  | Existing Plus Background Plus Project Option \#1 | 108 | 274 | 552 | 391 |
|  |  | Existing Plus Background Plus Project Option \#2 | 108 | 274 | 677 | 495 |
|  |  | Cumulative Plus Project Option \#1 | 119 | 324 | 707 | 460 |
|  |  | Cumulative Plus Project Option \#2 | 119 | 324 | 826 | 598 |

Note: The distance from the intersection of El Camino Real/Pastoria Avenue to the mid-block driveway is 350 feet.
Queue lengths shown in bold represent queues that exceed the available storage.
The queues for the westbound approach show that it would be difficult for vehicles to exit the midblock driveway and move over to the westbound left turn lane. The $95^{\text {th }}$ percentile westbound left turn queue at Pastoria Avenue/El Camino Real essentially uses the entire 290 -foot and 335 -foot left turn lane storage pockets in the PM peak hour for all scenarios. In addition, the $95^{\text {th }}$ percentile westbound through queue at Pastoria Avenue/El Camino Real extends at least 375 feet back from Pastoria Avenue, which would block vehicles exiting the mid-block driveway from getting into the westbound left turn lanes. Therefore, vehicles exiting this parking garage should be encouraged to use the driveway on Pastoria Avenue to go eastbound on El Camino Real.

In addition, a SimTraffic analysis was completed to confirm these conclusions regarding the southbound queues at the intersection of El Camino Real/Pastoria Avenue. The SimTraffic analysis showed that the 95th percentile westbound left turn queues will be 224 feet and the westbound through queues will be 267 feet for the Existing Plus Project Option \#2 PM peak hour. Although these queues are less than the 350 feet between the mid-block driveway and Pastoria Avenue/EI Camino Real, it would still be very difficult for vehicles to cross the three westbound through lanes from the mid-block driveway and enter into the westbound left turn lane at Pastoria Avenue/El Camino Real that has a 224 -foot queue. Since this scenario should have less volumes than the Existing Plus Background and Cumulative volumes, it is assumed that these same issues would occur in those scenarios.

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## SITE CIRCULATION AND ACCESS

The site circulation and access was reviewed for each of the two options. For Option \#1, the key difference is that Olive Avenue would remain unchanged from the existing conditions. This is beneficial to the site access and circulation because it allows for vehicles to enter and exit the project site from the signalized intersection of W Olive Avenue/Mathilda Avenue. Since a high proportion of project trips will be going to or coming from Mathilda Avenue to the north, a signal control is needed to make the left turn onto northbound Mathilda Avenue. The intersection of W Olive Avenue/Mathilda Avenue also provides sufficient time for vehicles that are destined for eastbound El Camino Real in the PM peak hour. As shown in the Synchro and SimTraffic analysis, it is difficult for vehicles to get to the southbound left turn lane at the intersection of Mathilda Avenue/El Camino Real from just the mid-block driveway at All America Way.

For Option \#2, the major difference is the removal of Olive Avenue between Pastoria Avenue and Charles Street. This change from the existing conditions has made it difficult for vehicles exiting the project site to go northbound on Mathilda Avenue and eastbound on El Camino Real in the PM peak hour. Since the proposed layout for Option \#2 restricts access to the intersection of W Olive Avenue/Mathilda Avenue, vehicles have to exit at mid-block driveways and make U-turns to get to their final destinations. Below is a list that summarizes the key circulation issues:

1) Vehicles cannot exit the mid-block driveway at All America Way and go northbound on Mathilda Avenue or eastbound on El Camino Real. In addition to the long queues, the high speeds and high volumes make it difficult for vehicles to find an acceptable gap to cross three lanes to enter into the left turn lane.
2) Vehicles cannot exit the mid-block driveway on El Camino Real (near the southwest parking garage) and go eastbound on El Camino Real or to Mathilda Avenue. In addition to the long queues, the high speeds and high volumes make it difficult for vehicles to find an acceptable gap to cross three lanes to enter into the left turn lane.
3) The driveway on Pastoria Avenue between the Library and the southwest parking garage is offset from the intersection of Pastoria Avenue/W Olive Avenue. This may result in sight distance issues for the driveway. There also exists a raised median on Pastoria Avenue just south of W Olive Avenue that would prevent vehicles from accessing this driveway.
4) Access to the parking garage underneath City Hall is provided at the intersection of W Olive Avenue/Charles Street. This access is beneficial because it allows access to Mathilda Avenue at the signalized intersection of Mathilda Avenue/W Olive Avenue.

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## SUMMARY OF IMPACTS AND MITIGATIONS

The proposed project is expected to generate significant impacts at the study intersections in multiple scenarios. Table 14 summarizes the significant impacts.

Table 14 - Impact Summary

| \# | Intersection | Scenario |
| :---: | :---: | :---: |
| \#23 | Mary Avenue / <br> W Olive Avenue | Existing Plus Project Option \#1-AM Peak |
|  |  | Existing Plus Project Option \#2 - AM Peak |
|  |  | Existing Plus Background Plus Project Option \#1-AM Peak |
|  |  | Existing Plus Background Plus Project Option \#2-AM Peak |
|  |  | Cumulative Plus Project Option \#1-AM and PM Peaks |
|  |  | Cumulative Plus Project Option \#1-AM and PM Peaks |

The intersection of Mary Avenue/W Olive Avenue is significantly impacted in each project scenario for the AM peak hour and impacted in the Cumulative Plus Project Options \#1 and \#2 for the PM peak hour. The intersection does not meet the signal warrant for any scenario. The main issue for this intersection is that the eastbound and westbound approaches are one lane and there are vehicles that want to make a left turn or through movement across Mary Avenue. Mary Avenue has a high volume going northbound and southbound, making it difficult for drivers to find an acceptable gap. Therefore, the recommended mitigation is to restrict left turns and through movements during the peak hours. This significantly reduces the side-street delay to an acceptable LOS C in the Cumulative Plus Project condition in the PM peak hour (i.e. the most congested scenario). It should be noted that motorists can use the adjacent signalized intersections of Mary Avenue/El Camino Real and Mary Avenue/W lowa Avenue to make the needed left turn movements.

## CONCLUSIONS

The preliminary results of the traffic evaluation demonstrated that the proposed project would result in significant impacts at the intersection of Mary Avenue/W Olive Avenue. This can be mitigated by restricting the westbound and eastbound approaches on Olive Ave to be right turns only during the AM and PM peak periods.

The preliminary Synchro analysis showed that the southbound queues at the intersection of El Camino Real/Mathilda Avenue would block vehicles exiting the All America Way driveway from entering the southbound left turn lane at El Camino Real/Mathilda Avenue. This would then restrict vehicles from going northbound on Mathilda Avenue since Olive Avenue is partially closed.

The preliminary Synchro analysis also showed that the westbound queues at the intersection of El Camino Real/Pastoria Avenue would block vehicles exiting the mid-block driveway from entering the westbound left turn lane at El Camino Real/Pastoria Avenue.

## Kimley»Horn

The key site circulation and access issues for Option \#2 are:

1) Vehicles cannot exit the mid-block driveway at All America Way and go northbound on Mathilda Avenue or eastbound on El Camino Real. In addition to the long queues, the high speeds and high volumes make it difficult for vehicles to find an acceptable gap to cross three lanes to enter into the left turn lane.
2) Vehicles cannot exit the mid-block driveway on El Camino Real (near the southwest parking garage) and go eastbound on El Camino Real or to Mathilda Avenue. In addition to the long queues, the high speeds and high volumes make it difficult for vehicles to find an acceptable gap to cross three lanes to enter into the left turn lane.
3) The driveway on Pastoria Avenue between the Library and the southwest parking garage is offset from the intersection of Pastoria Avenue/W Olive Avenue. This may result in sight distance issues for the driveway. There also exists a raised median on Pastoria Avenue just south of W Olive Avenue that would prevent vehicles from accessing this driveway.
4) Access to the parking garage underneath City Hall is provided at the intersection of W Olive Avenue/Charles Street. This access is beneficial because it allows access to Mathilda Avenue at the signalized intersection of Mathilda Avenue/W Olive Avenue.

Sincerely,


Ben Huie, P.E.
California Professional Engineer \#C76682

Attachments:
LOS Outputs


[^0]:    Source: City of Sunnyvale

